Environmental geological assessment of a Solid Waste Disposal site: a case in Ilorin, Southwestern, Nigeria.

Olusegun Omoniyi IGE¹, Olufemi OGUNSANWO²

1,2. Department of Geology and Mineral Sciences, University of Ilorin, Ilorin, Nigeria, P.M.B 1515, Ilorin, Kwara State Email: vickyige2002@yahoo.com, femiogunsanwo2003@yahoo.com

Abstract: This study was carried out to evaluate the effects of leachate from open landfill in the Ita-Amo area of Ilorin, southwestern Nigeria. It is also aimed at determining the possibility of upgrading the site to a modern waste containment facility (Sanitary Landfill) by evaluating the geotechnical properties of soils obtained over the site. A total of ten water samples and four representative soil samples were analyzed. Results of water analyses showed that the PH range from 6.6 to 7.1; Electrical Conductivity range from 107µs/cm to 241µs/cm; TDS range from 61mg/l to 971mg/l; Ca²⁺ range from 7.4mg/l to 64.0mg/l, Mg²⁺ range from 17.0mg/l to 61.0mg/l; HCO₃ range from 93.5mg/l to 250.0 mg/l; SO_4^{2-} range from 98.0 mg/l to 1400.0 mg/l; Cl range from 56.4 mg/l to 880.0 mg/l during the wet season. In the dry season, the concentration of ca²⁺ range from 41.3mg/l to 117.2mg/l; Mg²⁺ from 50.0mg/l to 100.4mg/l, K⁺ range from 2.1mg/l to 14.0mg/l; HCO₃ range from 91.8mg/l to 234.0mg/l; SO₄ range from 91.7mg/l to 1730.0mg/l; and Cl⁻ range from 43.7mg/l to 1347.0mg/l. The concentrations of Pb range from 41.2µg/l to $53.4 \mu g/l$ and $40.8 \mu g/l$ to $61.7 \mu g/l$; Zn^{2+} range from $57.0 \mu g/l$ to $60.0 \mu g/l$ and $15.0 \mu g/l$ to $67.1 \mu g/l$ at wet dry season respectively while Ni was 30.3µg/l as measured in the wet season only. Generally, the concentrations of these parameters are higher than WHO recommendations and decrease away from the centre of the waste dumpsite which is suggesting anthropogenic influence on the water chemistry. Geotechnically, the soils have 0.75% gravel, 32% sand, 48% clay and 19% silt in a compactible sandyclay soil. The result of the Atterberg limits tests showed that the soils are absolutely inorganic clay of low plasticity and average clay activity value of 0.39 which is suggesting nonreactive kaolinitic clay. The dry density of the soils are 1.80t/m³ and 2.1t/m³ when compacted at standard and modified Proctor energies respectively while the coefficient of permeability of the soils are in the order of 1x10⁻⁹m/s and 1x10⁻¹¹m/s respectively. These results are found favorably compared to recommendations of several researchers. Thus, the soils satisfied the requirements for use as mineral seals in sanitary landfills. The higher energy of compaction (Modified Proctor) is recommended because it offered lower values of coefficient of permeability. [Nature and Science. 2009;1(6):53-62]. (ISSN: 1545-0740).

Keywords: Sanitary Landfill; Leachate; Coefficient of Permeability; Mineral Seals; Solid waste

1. Introduction

Human population is increasing on daily basis, so is the corresponding quantity of waste contending for space with man and its effect impairing the quality of the environment. It is therefore very common to find waste dumps within built-up areas and cities in bags along roads and streets. Attempts by Nigeria government, groups and individual to check these problems include composting, open burning and river dump of refuse. These attempts had severally failed because of their inadequacies (Ige, 2003 and Asiwaju-Bello, 2004).

The city of Ilorin falls into southwestern (Figure 1) and Northcentral on geological and political classifications respectively the capital town

of kwara state, Nigeria. It has a total land coverage of over 400km² (Africa Atlases, 2007) and a population estimate of 2,185,494 people (NPC, 2007) which are responsible for the generation of waste often deposited in open spaces, river banks, road side etc. thereby degrading the quality of the environment. In attempt to alleviate environmental pollution within the city, three (3) final waste disposal sites were prepared (unengineered) and located strategically at the outskirt of the city (Figure 2). However, the selection, design, construction and operational activities of these sites did not consider geology and impacts on the adjacent environment. One of the deposal sites, along Ilorin -

peke village has been investigated as presented in this study.

This study is therefore aimed at assessing the effects of leahate that is generated from dumpsite on the shallow groundwater of the study area to

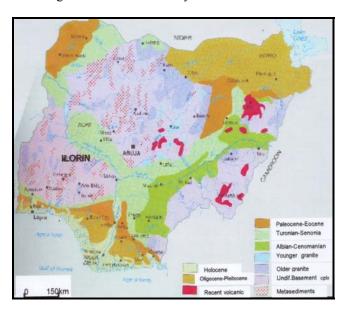


Figure 1: Geological map of Nigeria showing Ilorin (Modified from Africa Atlases, 2007)

2. Study area

The study area (Ita-Amo waste disposal site, Ilorin) is located within latitude 8° 25° N and 8° 30° N and longitude 4° 20° E and 4° 30°E. The approximate area extent of the dumpsite is 3.63x10⁶m² with average dump thickness of about 7.7m. The site inhabits and still occupying several farm land area. Geologically, the area lies in the Precambrian Basement complex of southwestern Nigeria and is underlain by rock of metamorphic and igneous type. However, migmatite predominantly underlies the waste dumpsite and characterized by weathered regolith which vary in thickness from place to place. The hydrologic setting of the area studied is typical of what is obtained in other Basement complex area

ascertain the extent of influence on the qualities of such water resources. The geotechnical and environmental issues were also investigated for the possible upgrading of the site to a modern solid waste containment facility such as sanitary landfill.

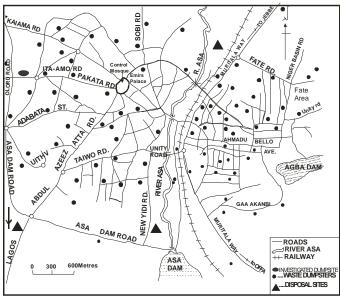


Figure 2: The Locations of waste dumpsters and final disposal site in Ilorin.

where the availability of water is a function of the presence of thick-little clay overburden material and presence of water filled joints, fracture or faults within the fresh Basement rocks. The humid tropical climate of Ilorin has particularly encourage relatively deep weathering of the near surface rocks to produce porous and permeable material that allows groundwater accumulation as shallow aquifer which is recharged principally through infiltration of rainwater. At the investigated dumpsite, the waste leachate may also infiltrate to pollute the shallow ground water.

3. Material and Methods

10 water samples were used in this study. 4 shallow water wells were dug around the solid waste deposition centre (1within the dump, 3 outside the dump and a river water sample). The well within the dumpsite is labeled 1while wells at distances "devoid of hydraulic interconnectivity" away from dumpsite and the river water sample and labeled 2,3,4 and 5 (50m, 70m, 120m and 327m away from dumpsite) respectively. Each of the well was dugged below the groundwater table to provide column for water accumulation. The water sampling was done in the well and dry season to monitor pollution (if any) with respect to season. Each of the water samples was collected into 2-litre container for the determination of anion concentration while 1-litre was used for cation concentration determination. The cation containers were acidified with 1-2 drops of HCl acid to prevent adhering to the surface of the container. The sensitive physical parameter such as total dissolve solid (TDS).

Electrical conductivity (Ec) and P^H were determined in the field using portable digital conductivity and PH meters also measure on the field are colour and turbidity. The analyses of the chemical constituents of the water sample were carried out at the water laboratory UNICEF office in Ilorin, Nigeria. Major cations (Mg²⁺, Na⁺, Ca²⁺, K⁺) and anions (HCO₃⁻, SO₄²⁻, Cl⁻) were analyzed using the flame photometry and atomic absorption photometry method while calorimetric, gravimetric and titrimetric methods were used for the determination of the anions.

Also four soils were recovered from the shallow wells at different depth within the lateritic zone. The variation in depth of soil sample was necessary to know the geotechnical properties of the whole laterite zone which may be useful as a mineral seals in the construction of modern waste containment facility (sanitary landfill). All the soil samples were analyzed with respect to their grain size distribution, Atterberg limit, moisture contentdensity relationship and the coefficient of permeability (K) characteristics at the soil laboratory of the Yaba College of technology, Lagos, Nigeria according to the BS 1377: 1990 standard. The results obtained were later compared recommendation of several previous researchers and regulatory agencies.

4. Results and Discussions

4.1 Physical Properties of Water Samples

Summary of the physical properties (and some site characteristics) of water sample is presented in Table 1. The P^H value range between 6.8 - 7.9 at both seasons. This range falls within the acceptable boundary of (WHO, 1993) for water usage in drinking. The colour value range between 15Hazen

- 61Hazen units in the wet season and 50Hazen - 92Hazen in dry season with highest value observed at well 1. Turbidity is higher in dry season than in wet season, probably due to dilution activities in the wet season, and decrease away from the dumpsite area (Table 1).

Table1: Physical Properties of Water Sample and other site Characteristics.

| Properties | W1 | W2 | W3 | W4 | SW |
|---------------------------|------|------|------|------|----|
| Static water level (m) | | | | | - |
| , , | 5.71 | 6.30 | 6.57 | 4.9 | |
| Depth of Well (m) | | | | | _ |
| . , | 6.47 | 6.47 | 7.15 | 6.71 | |
| Thickness of horizon(m) | | | | | - |
| | 3.90 | 3.58 | 3.84 | 3.79 | |
| Depth of soil sampling(m) | | | | | - |
| 1 0 , | 1.15 | 2.70 | 4.50 | 3.10 | |

W1-4: Well Number

SW: Surface Water

4.2 Chemical Properties of Water Samples

The relative abundance of the elements is shown in Table 2. The calcium ion concentration range from 7.4 mg/l - 64mg/l and 41mg/l -121.4mg/l during wet and dry season respectively. While the magnesium ion concentration range from 17.0mg/l -61.0 mg/l and 17.0mg/l-100.4mg/l during wet and dry season respectively. Concentration of sodium ion range from 12.0 mg/l - 57.5 mg/l in the wet season, and 23.0 mg/l to 109.4mg/l in the dry season while potassium ion concentration range from 0.78mg/l -2.70mg/l and 1.30mg/l -14.0 mg/l during wet and dry season respectively. Calcium concentration falls below the maximum permissible level (MPL) of their concentration in water to be useful for drinking purpose (WHO, 1993). This may be due to little or no interaction of water with the underlying basement rock (the principal source of calcium and magnesium ion) since all water samples were collected within vadose zone. However, concentrations above allowable level (AL) were noticed at wells1, 2 and 3 in dry season and decreases with distance away from the centre of the dumpsite (Figures 3-12).

For the anions, bicarbonate concentrations range from 93.5mg/l - 250mg/l in the wet season and from 91.8mg/l - 234.0mg/l in dry season. The sulphate range from 98.0-1400.0mg/l and 91.7mg/l -1730.4mg/l in the wet and dry season respectively. The chloride ion concentration range from 56.4mg/l -880.0mg/l ion in the wet and from 43.7mg/l -1347.0mg/l in the dry season. The nitrate ion concentration (where determined) range from 17.0mg/l - 31.0mg/l and 20.1mg/l-53.4mg/l at the two season respectively. All the parameters (anions) were observed to be above MPL at the wells (1-4) during the two seasons and decrease away in concentration from dumpsite centre (Table 2). This may be connected to the liquification and leaching of decomposed dominantly domestic, industrial and commercial waste within the dumpsite as a result of the action of heat, pressure and presence of water. Generally, low concentrations of these parameters were observed which may be due to pollutant filtering capability of the underlying soil, season and age of dumpsite.

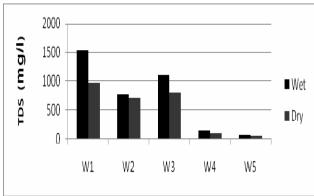


Figure 3. Concentration of Total Dissolved solid (TDS)

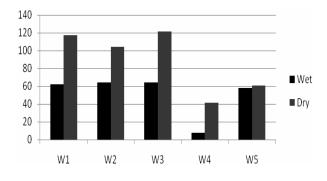


Figure 4. Variation of calcium concentration in the wells

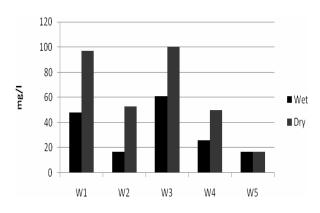


Figure 5. Variation of magnesium ion concentration in the wells

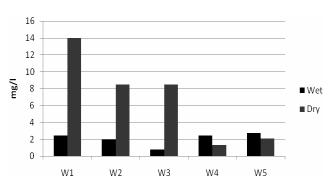


Figure 6. Variation of Potassium ion concentration in the wells

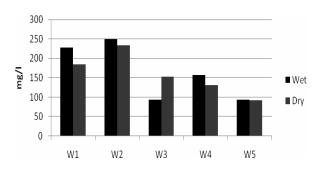


Figure 7. Variation of Bicarbonates ion concentration in the wells

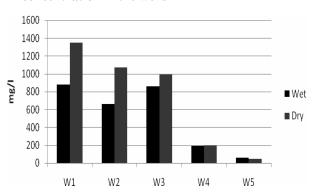


Figure 9. Variation of Chloride ion concentration in the wells

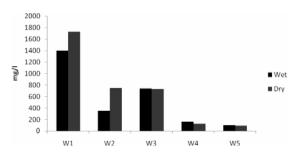


Figure 8. Variation of Sulphate ion concentration in the wells

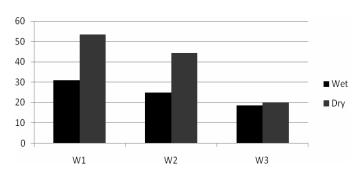


Figure 10. Variation of Nitrate ion concentration in the wells

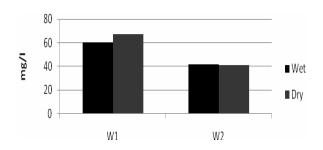


Figure 11. Variation of Zinc ion Concentration in the wells

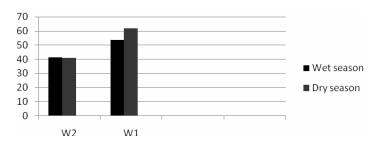


Figure 12. Variation of Lead ion concentration in the wells

T able 2: Chemical Properties of Water Samples

| Parameters | Wet | Dry | Wet | Dry | Wet | Dry | Wet | Dry | Wet | Dry | AL | MPL |
|-------------------|-------|--------|-------|--------|-------|-------|-------|-------|------|------|----------|----------|
| PH | 7.1 | 6.6 | 7.2 | 6.8 | 7.6 | 7.1 | 7.4 | 7.1 | 7.9 | 7.0 | - | |
| Ec (μs/cm) | 3320 | 2411 | 1585 | 1034 | 2360 | 1917 | 500 | 439 | 160 | 107 | - | 400 |
| TDS (mg/l) | 1530 | 971 | 785 | 704 | 1110 | 816 | 142 | 101 | 74 | 61 | - | 1000 |
| Calcium (mg/l) | 62.0 | 117.2 | 64.0 | 104.4 | 64.0 | 121.4 | 7.4 | 41.3 | 58.0 | 61.0 | 75.0 | 200.0 |
| Magnesium(mg/l) | 48.0 | 97.2 | 17.0 | 53.0 | 61.0 | 100.4 | 26.0 | 50.0 | 17.0 | 17.0 | 50.0 | 150.0 |
| Sodium (mg/l) | 12.0 | 49.4 | 57.5 | 109.4 | 12.0 | 61.1 | 23.0 | 27.0 | 25.0 | 23.0 | 20.0 | 200.2 |
| Potassium (mg/l) | 2.4 | 14.0 | 1.95 | 8.5 | 0.78 | 8.5 | 2.4 | 1.3 | 2.7 | 2.1 | 10.0 | 12.0 |
| Bicarbonate(mg/l) | 228.1 | 184.2 | 250.0 | 234.0 | 93.5 | 153.3 | 157.4 | 131.0 | 94.1 | 91.8 | variable | variable |
| Sulphate (mg/l) | 1400 | 1730.4 | 350.0 | 742.7 | 740.1 | 733.6 | 160.0 | 120.0 | 98.0 | 91.7 | 250.0 | 400 |
| Chloride (mg/l) | 880.0 | 1347.0 | 664.1 | 1074.1 | 860.0 | 994.4 | 193.9 | 200.7 | 56.4 | 43.7 | 250.0 | 600 |
| Nitrate (mg/l) | 31.0 | 53.4 | 24.7 | 44.4 | 18.4 | 20.0 | ND | ND | 17.0 | 20.1 | 25.0 | 50.0 |
| Zinc (µg/l) | 60.0 | 67.1 | 57.2 | 61.0 | ND | 15.4 | ND | ND | - | - | - | 50 |
| Lead (µg/l) | 53.4 | 61.7 | 41.2 | 40.8 | ND | ND | ND | ND | - | - | - | 50 |
| Nickel (µg/l) | 30.3 | 33.1 | ND | 17.4 | ND | ND | ND | ND | - | - | - | 50 |
| Colour (Hazen) | 61.4 | 92.3 | 53.4 | 68.1 | 51.4 | 60.7 | 15.5 | 50.6 | 31.4 | 56.1 | 5 | 50 |
| Turbidity | 21.7 | 28.2 | 17.0 | 33.9 | 9.3 | 20.3 | 7.7 | 7.3 | 12.8 | 7.2 | - | |

5.0 GEOTECHNICAL PROPERTIES OF SOIL

Several limits have been proposed by various researchers with respect to geotechnical properties of soils to be useful as barriers in landfills. Such limits

5.1 Grain Size Distribution

In the soils investigated the largest grain has diameter ≤ 6.3 mm. This is very small compare to 63mm suggested by ÖNORM S2074 (1990) and less than 50mm suggested by Daniel 1993. The percentage of clay contained in the soil range between 41% - 51% Table 3. These values are much higher than 15% proposed by ÖNORM S 2074 (1990) and less than 30% suggested by Daniel1993, Bagchi(1994), Benson *et al* (1994), Rowe (2005) and

have been compared with the results of the investigated parameters and presented as follow

Mohammedzein (1998). Oeltzeshner (1992) preferred soils with clay fraction of greater than 20%.

The percentage of gravel recommended by Daniel 1993, Bagchi (1994) and Rowe (1995) is less or equal 30% of the soil mass. The highest proportion of gravel from the investigated soil sample is 2% with an average value of 0.75% over the whole area. The specific gravity also ranges between 2.61% and 2.69% which is better than 2.2 recommended by ÖNORM S2074 (1990).

| Table 3: Grain size analysis of soil sample | es |
|---|----|
|---|----|

| | | | | | Grain | | | | |
|-----|------|----------|----------------------------|----------|-------|------|------|------|------|
| | Well | | Natural | Specific | Size | Sand | Clay | Silt | Fine |
| S/N | No | Depth(m) | Density(t/m ³) | Gravity | (%) | (%) | (%) | (%) | (%) |
| 1 | W1 | 1.15 | 2.04 | 2.69 | 2 | 28 | 48 | 22 | 70 |
| 2 | W2 | 2.70 | 1.67 | 2.64 | 1 | 34 | 45 | 20 | 65 |
| 3 | W3 | 4.50 | 1.60 | 2.63 | 0 | 28 | 51 | 2` | 72 |
| 4 | W4 | 3.10 | 1.91 | 6.61 | 0 | 36 | 49 | 15 | 64 |
| | | Average | 1.81 | 2.64 | 0.75 | 32 | 48.3 | 19 | 68 |

5.2 Atterberg Consistency Limits

The results of the liquid limits tests (L_I) for the soil range between 35.34% and 40.56% while the index of plasticity (Ip) range between 17.15% and 20.55% (Table 4). These are higher than recommendations of several previous workers. From the results, the Casangrande plasticity chart was plotted for all the soils. All the soil samples fall within the inorganic clay of intermediate plasticity (**Fig. 13**). This is a good result when compared to the recommendations of Bagchi (1994), Hughes *et al*

(2005) and Jagger and Ash (2008). Clay activities (A_C) of the soils were also evaluated to determine the reactivity of the soil. The values range between 0.35 and 0.40. It therefore suggested that the soil contain essentially kaolinitic clay mineral type which are non-reactive and non-expansive (Lambe 1951, Bagchi 1994 and Withlow (1998). They are less attack by chemical and withstand volumetric shrinkage (Taha and Kabir, 2006).

Table 4: Atterberg Consistency limits of the soil samples

| Symbol | WL (%) | WP (%) | IP (%) | Plots on Plasticity chart | Ac |
|---------|--------|--------|--------|---------------------------|------|
| W1 | 35.34 | 18.19 | 17.15 | CL | 0.35 |
| W2 | 39.14 | 20.80 | 18.34 | CI | 0.41 |
| W3 | 39.80 | 21.31 | 18.49 | CI | 0.39 |
| W4 | 40.56 | 20.01 | 20.55 | CI | 0.42 |
| Average | 38.71 | 20.00 | 18.63 | CI | 0.39 |

KEY: WL = Liquid Limit IP = Index of Plasticity WP = Plastic Limit Ac = Activity of Clay

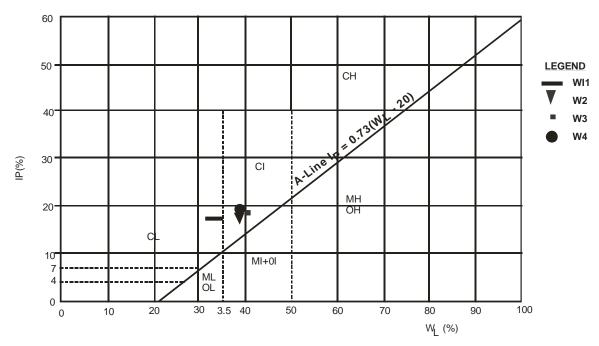


Fig.13: Position of soil samples on the Casagrande's Plasticitychart.

5.3 Moisture Content - Density Relationship

The results of compaction tests carried out at different energies of compaction to obtain the soils optimum moisture content (OMC) and the corresponding maximum dry density (MDD) is presented in the Table 5. Benson and Trast (1995) reported that the coefficient of permeability is sensitive to compactive effort and molding water content. The soil MDD values for standard Proctor energy of compaction range between 1.77t/m³ and

5.4 Coefficient of Permeability (k)

The coefficient of permeability is the key parameter affecting most soils to be useful as barrier in landfill (Benson, 1990). Thus great attention is focused at ensuring a low permeability is achieved. Several investigators and waste management agencies have recommended 1x10⁻⁹m/s as the minimum allowable value for soil to be useful for this purpose. From table 5, values lower than recommendation of several authors (e.g Mark 2002; Joyce 2003; Fred and Anne 2005 and Jagger and Ash, 2008) were obtained from all the soil investigated with both standard and modified Proctor

1.84t/m³ while MDD at modified Proctor energy range between 1.9t/m³ and 2.2t/m³. These values are higher than 1.7t/m³ stipulated in ÖNORM S 2074 (1990). They are also better than 1.45t/m³ (standard Proctor) and 1.64t/m³ (modified Proctor) of MDD recommended by kabir and Taha (2006) for soils produced from Basement complex rocks to be useful as barrier in landfills.

energies. It was also observed that the coefficient of permeability decreases with increased compactive energy (Table 5). This is because there is a decrease in the frequency of pores resulting from the structural rearrangement of soil particle in the soil mass (Acar and Oliveri, 1990). For the purpose of sanitary landfill, the least achievable coefficient of permeability on the field is preferred. Therefore the higher energy (modified Proctor) of compaction is recommended.

| Table 5: Maximum Dry Density and Coefficient of Permeability of the son Samples. | | | | | | | | | |
|--|------------------|-----------|----------|-----------|---------------------------|----------------------------------|--|--|--|
| WELL | STANDARD PROCTOR | | MODIFIED | | COEFFICIENT OF | | | | |
| SYMBOL | | | PROCTOR | | PREMEABILITY(K)cm/s | | | | |
| | OMC (%) MDD | | OMC (%) | MDD | STANDARD | MODIFIED | | | |
| | | (t/m^3) | | (t/m^3) | PROCTOR | PROCTOR | | | |
| W1 | 13.4 | 1.84 | 10.8 | 2.2 | 1.1x10 ⁻⁹ m/s | $3.4x10^{-11}$ m/s | | | |
| W2 | 14.0 | 1.78 | 10.3 | 1.9 | $4.0x10^{-9} \text{ m/s}$ | $5.1 \times 10^{-11} \text{m/s}$ | | | |
| W3 | 14.2 | 1.77 | 10.0 | 2.1 | 5.3x10 ⁻⁸ m/s | $3.6 \times 10^{-11} \text{m/s}$ | | | |
| W4 | 16.7 | 1.80 | 12.5 | 2.0 | 3.7x10 ⁻⁹ m/s | 2.3x10 ⁻¹¹ m/s | | | |
| AVERAGE | 14.6 | 1.80 | 10.9 | 2.1 | 3.53x10 ⁻⁸ m/s | $1.6 \times 10^{-11} \text{m/s}$ | | | |

Table 5: Maximum Dry Density and Coefficient of Permeability of the soil Samples.

OMC = Optimum Moisture Content

MDD = Maximum Dry Density

6. Conclusion

The following conclusions were made on the hydrogeological and geotechnical evaluation of the Ita-amo waste dumpsite in, Ilorin, Nigeria.

The physical properties of the water samples are quite lower than the allowable recommended by the WHO for drinking and irrigation purpose. However, there is seasonal influence on the cationic concentration in the dry season with wells 1, 2 and 3 having concentration above the WHO permissible level. Also, Liquification and leaching activities of the deposited large volume of domestic and industrial waste at site may have been responsible for the high increase in the ionic concentration at both seasons. Ionic concentration decreases with increase distance away from the waste dump centre.

The overall engineering characteristics of the soil samples recovered from test pits, irrespective of the depth of recovery, show that the soils are inorganic clay with low to medium plasticity. Generally, these types of soils possess desirable characteristics to minimize hydraulic conductivity of compacted soils. The indices properties (liquid limit, plastic limit, percentage fine, percentage gravel, activity etc) of the soil samples satisfy the basic requirements as barrier materials in landfills. They are inactive clayey soils. Thus, the soils will be less affected by waste chemical and also less susceptible to shrinkage. The soils have hydraulic conductivity of less than 1×10^{-7} cm/s when compacted with both modified and standard Proctor compaction efforts.

This result compared favorably with the recommendations of several researchers. Also higher energy of compaction is recommended because it gives lower values of coefficient of permeability for the compacted soils.

Correspondence to:

Olusegun Omoniyi IGE Leturer,

Department of Geology and Mineral Sciences, University of Ilorin, P. M. B. 1515, Ilorin, Nigria.

Telephone: +234-803-676-2252 Email: vickyige2002@yahoo.com

REFERENCES

- [1] Acar Y, Oliveri I. Pore Fluid Effects on the fabric and Hydraulic Conductivity of Laboratory Compacted Clay. United State Transportation Research Record. (1989). 12 (19): 144-159.
- [2] Africa Atlasses. Atlas of Nigeria. 2nd Editon. 2007. 156p.
- [3] Ash JS, Jagger DE. Geotechnical considerations at landfill site. Jagger. Hims Ltd. www.jaggerhims.com, 2008. 27p.
- [4] Asiwaju- Bello Y A, Akande OO. Urban groundwater pollution: Case study of a Disposal sites in Lagos metropolis. Journal of Water Resources. 2004. 12, 22-26.
- [5] Bagchi A. Design, Construction, and Monitoring of landfills. 2nd Edition, A Wiley-Interscience Publisher. New York, U. S. A. 1994. 361p.
- [6] Benson CH, Daniel DE. The effect of clods on hydraulic Conductivity of compacted clay. Journal of Geotechnical Engineering. 1990:116(8):1237-1248.
- [7] Benson CH, Zhai H, Wang X. Estimating hydraulic conductivity of compacted clay liners. Journal of Geotechnical Engineering. 1994; 120(2): 336-387.

- [8] Benson CH, Trast MJ. Hydraulic conductivity of Thirteen compacted Clays. Clays and clay minerals. 1995; 43(6): 669-681.
- [9] Daniel DE. Clay Liners. In: Geotechnical Practice for Waste Disposal (Daniel D.E ed). Chapman Hall, London, U.K, 1993 137-163.
- [10] Kurian L. Landfill types and Liners system. Ohio State University Fact Sheet: www.ohioline.ag.ohio-state.edu. 2005;23p
- [11] Ige, OO. Impact of cultural and industrial waste on surface and shallow ground water along Asa river, Ilorin metropolis, Kwara State, Nigeria. University of Ilorin, Geology and Mineral Sciences, M.Sc., Unpublished Thesis, 2003; 108p.
- [12] Joyce MPE. A study of the merit and effectiveness of alternate liner Systems at Illinois landfills. A research paper submitted in fulfillment of house resolution 715. State of Illinois 92nd General Assembly. 2003; 46p.
- [13] Kabir MH Taha, T R. Sedimentary Residual soils as a hydraulic barrier in waste containment systems. 2nd International Conference on Advances in soft soil Engineering. Technology Putrajaya, Malaysia. 2003; 894-904.

- 14 Mark, Y. Geology and Geotechnical Investigation of the proposed anterbury Regional Landfill Kate Valley, North Canterbury Transwaste Canterbury Ltd. 2002; 147p.
- [15] Mohamedzein YA. Assessment of crushed Shales for use as compacted landfill liners. Journal of Engineering Geology. 2005; 80: 271-281
- [16] Oltzchner H. Anforderin an die Geologic, Hydrogeologie Und Geotechnik beim Bau von Deponie. In: THORME-KOZMIENSKY KJ (eds), Additictung Von Deponien and Altlastlen. E. F. Verlag für Gnergie und Umwelttechnik combit, Borlin. 1992; 53-82.
- [17] ÖNORMS 2074. Geotechnik in Deponiebau-Erdarbeiten. Osterrichisches
- Normungsinstitut, Wein. In: Ogunsanwo O. Geotechnical Investigation of some soils from Southwestern Nigeria for use as mineral seal in waste disposal landfills. Bulletin of IAEG, Paris. 1999; 54: 119-123.
- [18] Rowe Rk. Long-term performance of contaminant barrier systems. Geotechnique. 2005; 55(9): 631-678.
- [19] Withlow, R Basic Soil Mechanics. 3rd Edition. Longman Group Ltd. 1998; 557p.

8/2/2009